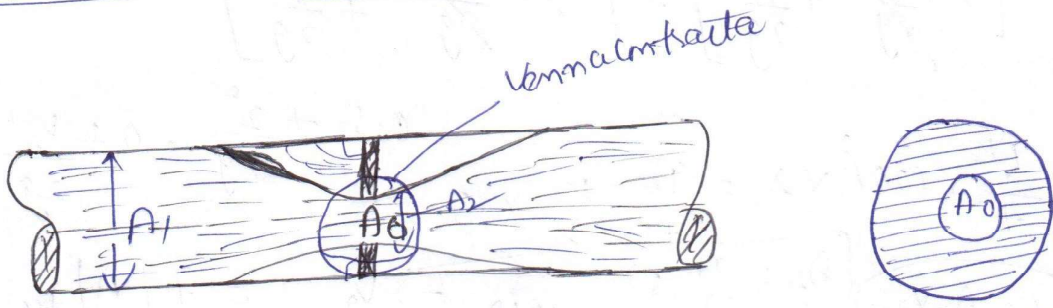


# ORIFICE METER :- (to measure flow rate)



Coefficient discharge  $C_d = \underline{\underline{0.63}}$

$$\frac{\pi}{\pi + 2}$$

→  $A_1$  = Area of main pipe

→  $A_0$  = Area of orifice

→  $A_2$  = The minimum area at vena contracta

$C_c$  = Coefficient of contraction

$$C_c = \frac{A_2}{A_0}$$

$$A_2 = A_0 \times C_c \quad \checkmark$$

$$Q = C_d \frac{A_1 A_2 \sqrt{2gh}}{\sqrt{A_1^2 - A_2^2}} \quad \checkmark$$

Q. 1) The measurement of water flow using a venturimeter with a coefficient of 0.98 and a constant of  $0.3 \text{ m}^{2.5}$  using mercury as manometric fluid. The difference in pressure head across venturi is estimated as 50 cm of Hg. Then the approximate flow rate will be \_\_\_\_\_

$$Q = C_d k \sqrt{H}$$

$$C_d = 0.98$$

$$k = 0.3 \frac{\text{m}^{2.5}}{\text{sec}}$$

$$H = \frac{\text{pressure difference}}{\text{density} \times g} = \frac{50 \text{ cm of Hg}}{\rho \times g}$$

$$g = 9.81$$

$$H = \frac{50 \text{ cm of Hg}}{\rho \times g} = 0.5 \text{ m}$$

$$Q = C_d \times k \times \sqrt{H}$$

$$Q = 0.98 \times 0.3 \times \sqrt{13.6 \times 0.5}$$

$$= 0.98 \times 0.3 \times 2.607$$

$$= \underline{\underline{0.76 \text{ m}^3/\text{sec}}}$$

(13.6) specific gravity

(2) Flow of water through circular pipe measured by using venturimeter with  $C_d = 0.98$  it replace an orifice meter with  $C_d = 0.63$  for the same flow parameters the ratio of pressure drop across venturi to orifice

$$C = \boxed{Q = C_d \cdot A \cdot \sqrt{2gH}} \rightarrow \Delta P$$

$\therefore C_d(\text{vent}) = 0.98$   
 $C_d(\text{orifice}) = 0.63$

$$\frac{\Delta P_{\text{vent}}}{\Delta P_{\text{orifice}}} = ?$$

$\rightarrow \Delta P = \sqrt{2gH} \rightarrow$  pressure head drop because

$\rightarrow \sqrt{\Delta P} \propto \frac{1}{C_d} \Rightarrow \Delta P \propto \frac{1}{C_d^2}$   
 $\Rightarrow \Delta P \propto \frac{1}{C_d^2}$

$\rightarrow \frac{\Delta P_{\text{vent}}}{\Delta P_{\text{orifice}}} = \frac{\frac{1}{C_d^2}}{\frac{1}{C_d^2}} = \frac{1}{C_{d, \text{vent}}^2} \times \frac{C_{d, \text{orifice}}^2}{1} = \frac{(0.63)^2}{(0.98)^2}$

(3) For flow of water through an inclined pipe ( $30^\circ$  inclination) with horizontal of  $30 \text{ cm}$  dia. well connected by a venturi of  $15 \text{ cm}$  dia using mercury as manometric fluid with reflection of  $50 \text{ cm}$  calculate the approximate flow rate considering the losses across venturi as 20% of its kinetic energy.

$A_1 = 30 \text{ cm dia}$   
 $A_2 = 15 \text{ cm dia}$

$C_m = \text{Hg} = 13.6$

$\rightarrow h = 50 \text{ cm}$

$h_1 = 20\%$  of kinetic energy of inlet

$0.2 \frac{V_1^2}{2g}$

$A_1 = \frac{\pi (30)^2}{4} = 706.8 \text{ cm}^2 \Rightarrow 7.068 \text{ m}^2$

$A_2 = \frac{\pi (15)^2}{4} = 176.7 \text{ cm}^2$



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$$Q = C_d \frac{A_1 A_2}{\sqrt{A_1^2 - A_2^2}} \sqrt{2g \left( \frac{h_m}{s_0} - 1 \right)}$$

$$d_1 = 30 \text{ cm} \rightarrow h_m = s_g = 13.6$$

$$d_2 = 15 \text{ cm} \rightarrow y = 50 \text{ cm}$$

$$h_L = 20.1 \text{ (s. E. of inlet)}$$

$$\rightarrow \underline{0.2} \frac{v^2}{2g}$$

$$A_1 = \frac{\pi \times (0.3)^2}{4} = 0.0707 \text{ m}^2$$

$$A_2 = \frac{A_1}{4} = 0.01767 \text{ m}^2$$

Take  $C_d = 1.0$  (approx)  
 $C_{ct} = 0.96$

$$Q = 1 \times \frac{0.0707 \times 0.01767}{\sqrt{(0.0707)^2 - (0.01767)^2}} \times \sqrt{2 \times 9.81 \times 0.5 \left[ \frac{13.6}{1.0} - 1 \right]}$$

$$\rightarrow Q = \underline{0.202 \text{ m}^3/\text{sec}}$$

(So = water)

$$\boxed{E_1 = E_2 + \text{loss}}$$

loss

$$\left[ z_1 + \frac{P}{\rho g} + \frac{v_1^2}{2g} \right]_1 = \left[ z_2 + \frac{P}{\rho g} + \frac{v_2^2}{2g} \right]_2 + \underline{0.2 \frac{v_1^2}{2g}}$$

$$= \left[ z_1 + \frac{P}{\rho g} \right]_1 - \left[ z_2 + \frac{P}{\rho g} \right]_2 = \frac{v_2^2}{2g} + 0.2 \frac{v_1^2}{2g} - \frac{v_1^2}{2g}$$

Vanturn head

$$= y \left( \frac{s_m}{s_0} - 1 \right)$$

$$= \left[ z_1 + \frac{P}{\rho g} \right]_1 - \left[ z_2 + \frac{P}{\rho g} \right]_2 = \frac{v_2^2}{2g} + 0.2 \frac{v_1^2}{2g} - \frac{v_1^2}{2g} = y \left[ \frac{s_m}{s_0} - 1 \right]$$

$$\frac{v_1^2}{2g} [0.2 - 1]$$

$$v_2^2 - \frac{0.8 v_1^2}{1} = 2g \left( \frac{S_M}{S_0} - 1 \right)$$

$$\frac{\left( \frac{Q}{A_2} \right)^2}{1} - \frac{0.8 Q^2}{(A_1)^2} = 2g \left( \frac{S_M}{S_0} - 1 \right)$$

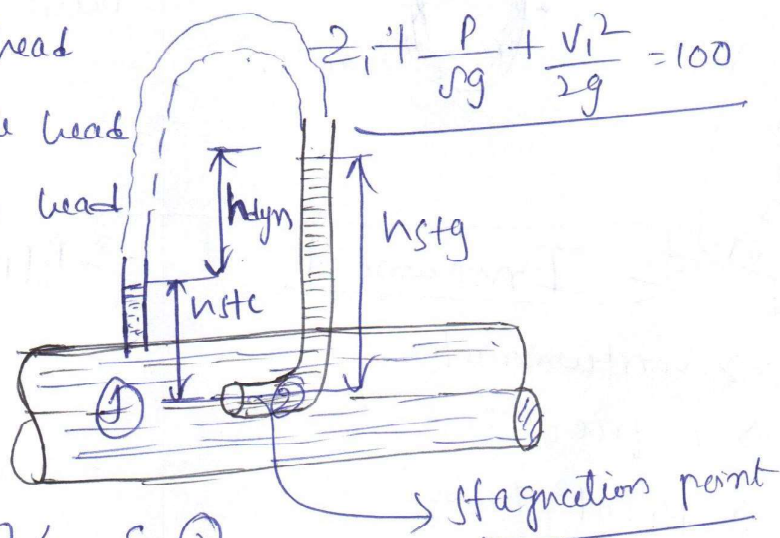
$$\sqrt{\frac{2g \left( \frac{S_M}{S_0} - 1 \right)}{\frac{1}{(A_2)^2} - \frac{0.8}{A_1^2}}} = \sqrt{\frac{0.5 \left[ \frac{13.6}{1.0} - 1 \right]}{\frac{1}{0.01767} - \frac{0.8}{0.0707}}}$$

$$Q = \underline{\underline{0.201 \text{ m}^3/\text{s}}}$$

Pitot tube := (To measure velocity)

Pitot tube measure

- (a) Velocity
- (b) Static pressure head
- (c) Dynamic pressure head
- (d) Stagnation pressure head



→ Apply Bern Eq (1) & (2)

$$\left[ z_1 + \frac{P}{\rho g} + \frac{v_1^2}{2g} \right] = \left[ z + \frac{P}{\rho g} + \frac{v_2^2}{2g} \right]$$

$$\underbrace{\left[ z_1 + \frac{P}{\rho g} \right]}_{h_{\text{static}}} + \frac{v_1^2}{2g} = \underbrace{\left[ z + \frac{P}{\rho g} \right]}_{h_{\text{stg}}} + \frac{v_2^2}{2g} \quad \rightarrow 0 \text{ (stg)}$$

$$\frac{v_1^2}{2g} = h_{stg} - h_{stc} = h_{dyn}$$

$C_c =$  Coefficient of contraction

$$v_{th} = \sqrt{2gh_{dyn}}$$

$\rightarrow C_v =$  Coefficient of velocity  $= \frac{v_{act}}{v_{the}}$

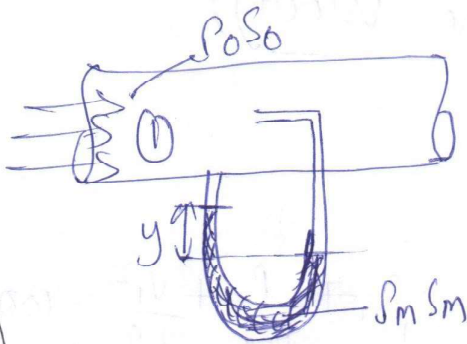
$$\rightarrow v_{act} = C_v \cdot \sqrt{2gh_{dyn}}$$

$$C_D = C_c \times C_v$$

$$h_{dyn} = h_{stg} - h_{stc}$$

Pitot static tube :-

To measure gas velocity  
 $\rightarrow$  I.C. flow  $[M < 0.4]$



$$v_{act} = C_v \sqrt{2 \cdot g \cdot h_{dyn}} = \sqrt{2g \cdot y \left[ \frac{\rho_m}{\rho_0} - 1 \right]}$$

$$h_{dyn} = y \left[ \frac{\rho_m}{\rho_0} - 1 \right] \left( \frac{\rho_m}{\rho_0} \right)$$

$$s = \frac{\rho_{air}}{\rho_w}$$

$$\begin{aligned} V &= IR \\ R &= \frac{PL}{A} \\ P &= VI \\ H &= I^2 RT \end{aligned}$$

Instrument

Application

- $\rightarrow$  Venturimeter
- $\rightarrow$  Orifice meter
- $\rightarrow$  Pitot tube  $\rightarrow$
- $\rightarrow$  Current meter  $\rightarrow$
- $\rightarrow$  Rotameter  $\rightarrow$
- $\rightarrow$  Hot wire Anemometer  $\rightarrow$
- $\rightarrow$  Hydrometer  $\rightarrow$
- $\rightarrow$  Hygrometer  $\rightarrow$

- (to measure)
- $\rightarrow$  Flow Rate  $\leftarrow (AP)$
- $\rightarrow$  velocity  $\leftarrow h_{dyn} \leftarrow h_{stg}$
- $\rightarrow$  velocity
- $\rightarrow$  Flow Rate
- $\rightarrow$  Turbulent vel. fluctuations  
 $\leftarrow$  unsteady flow
- $\rightarrow$  Relative density (sp. gravity) Buoyancy
- $\rightarrow$  Humidity (moisture)

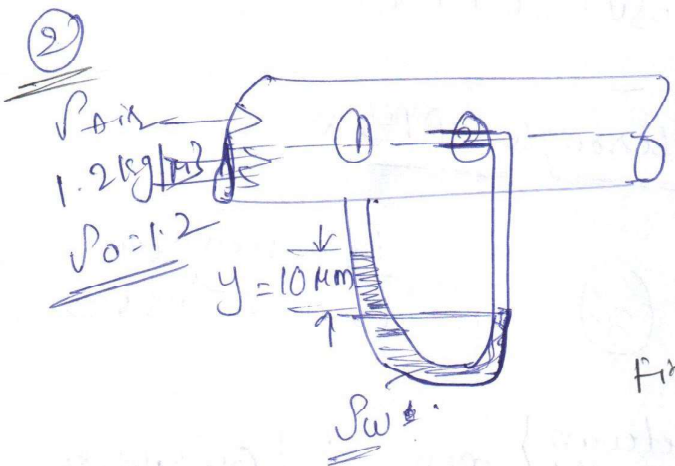


1) For measurement of velocity of air by using pitot static tube with coefficient 0.98. The stagnation pressure head was estimated as 3m at the static pressure 0.5m and the approximate velocity  $v = ?$

Given:  $C_v = 0.98$   
 $h_{stg} = 3.0 \text{ m}$   
 $h_{stc} = 0.5 \text{ m}$   
 $v = ?$

$$\begin{aligned} v &= C_v \sqrt{2g h_{dyn}} \\ &= 0.98 \sqrt{2 \times 9.81 \times 2.5} \\ &= \underline{6.8 \text{ m/s}} \quad \text{Ans} \checkmark \end{aligned}$$

$$\begin{aligned} \rightarrow h_{dyn} &= h_{stg} - h_{stc} \\ &= 3.0 - 0.5 \\ &= \underline{2.5 \text{ m}} \quad (h_{dyn}) \checkmark \end{aligned}$$



$v = ?$

$$v = C_v \sqrt{2g h_{dyn}}$$

Then  $C_v = 1.0$

$$\text{Find } v = C_v \sqrt{2g y \left( \frac{P_M}{P_0} - 1 \right)}$$

$$= 1.0 \sqrt{2 \times 9.81 \times 10 \times 10^{-3} \left[ \frac{1000}{1.2} - 1 \right]}$$

$$\underline{v = 12.8 \text{ m/s}}$$

3) For measurement of velocity of air by using pitot static tube with density  $1.2 \text{ kg/m}^3$  with manometric the difference 380 pas then the

velocity of air is found of using pitot static tube fluid with density  $800 \text{ kg/m}^3$  static & stagnation estimated approximate velocity of flow!

$\rho_{air} = 1.2 \text{ kg/m}^3$  ✓

$\rho_m = 800 \text{ kg/m}^3$

$P_{static} - P_{stg} = \rho_m h_{dyn} \Rightarrow P_{dyn} = 380 \text{ Pa} \cdot \text{N/m}^2 \Rightarrow P_{dyn} = 380 \text{ Pa} \cdot \text{N/m}^2$

$P_{static} - P_{stg} = P_{dyn} \quad \underline{v} = ?$

$h_{dyn} = \frac{P_{dyn}}{\rho \times g}$

$v = C_v \times \sqrt{2g h_{dyn}}$

$h_{dyn} = \frac{P_{dyn}}{\rho \cdot g}$

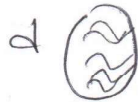
$v = 1.0 \sqrt{2 \times 9.81 \times \frac{380}{\rho \times g}}$

~~$v = 1.0 \sqrt{2 \times 9.81 \times \frac{380}{\rho \times g}}$~~

$v = 1.0 \sqrt{2 \times 9.81 \times \frac{380}{981 \times 1.2}} \Rightarrow v = 26 \text{ m/sec}$

Flow through pipes

$Re = \frac{F_i}{F_v} = \frac{\rho v L}{\mu}$



$\frac{F_i}{F_v} = \frac{\text{inertia force}}{\text{viscous force}}$

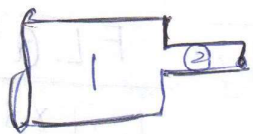
	<u>Circular pipe</u>	<u>Between parallel plates</u>	<u>open channel</u>	<u>Quiescent through soil</u>
<u>Laminar</u>	$Re < 2000$	$Re < 1000$	$Re < 500$	$Re < 1$
<u>Critical</u>	$Re \text{ critical } \approx 2300$			
<u>Transitional</u>	$2000 < Re < 4000$	$1000 < Re < 2000$	$500 < Re < 1000$	$1 < Re < 2$
<u>Turbulent</u>	$Re > 4000$	$Re > 2000$	$Re > 1000$	$Re > 2$

$E_1 = E_2 + h_{\text{loss}}$

major loss [80-90%] [in fraction]  
 minor loss [10-20%] Pipe cl variation External fitting

→ Minor loss :-

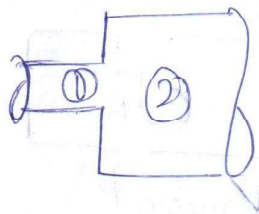
→ ①  $h_L$  sudden contraction :-



If  $C_c$  is not given

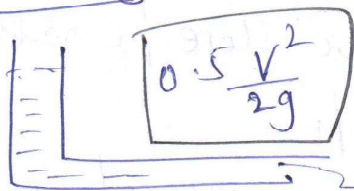
$$h_L = \left[ \frac{1}{C_c} - 1 \right]^2 \frac{v_2^2}{2g} \approx \frac{0.5 v_2^2}{2g}$$

→ ②  $h_L$  sudden expansion :-



$$h_L = \frac{[v_1 - v_2]^2}{2g}$$

→ ③  $h_L$  entry



$\therefore K = \text{loss coefficient}$

④  $h_L$  exit

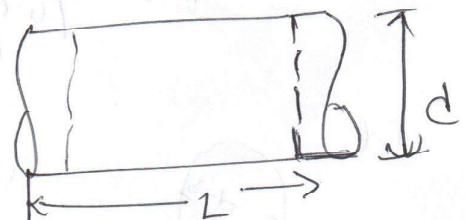
$$h_L = \frac{v^2}{2g}$$

$$h_L = K \left( \frac{v^2}{2g} \right)$$

① Friction loss :- ( $h_f$ ) :-

→ ① Darcy Weisbach's equation :-

$$h_f = \frac{4 f L V^2}{2g d}$$



$f = \text{friction coefficient}$   
 $= 0.005 \text{ to } 0.01$



Modified

Darcy weitchbaule

eg<sup>m</sup>

$$\Rightarrow \frac{FLV^2}{2gd}$$

$\rightarrow F = 4f =$  friction factor

0.02 to 0.04

$$\begin{aligned} Q &= A \cdot V \\ &= \frac{\pi d^2}{4} \cdot V \end{aligned}$$

$$\rightarrow h_f = \frac{F \cdot L \left( \frac{Q}{\frac{\pi d^2}{4}} \right)^2}{2 \cdot g \cdot d}$$

$$\Rightarrow \frac{F \cdot L \cdot \left( \frac{Q}{\frac{\pi d^2}{4}} \right)^2}{2 \cdot g \cdot d}$$

$$\Rightarrow \frac{FLQ^2}{12.1 d^5}$$

$$\Rightarrow h_f = \frac{FLQ^2}{12.1 d^5}$$

$\rightarrow f = \frac{64}{Re} \rightarrow$  laminar

$\rightarrow f = \frac{0.316}{(Re)^{1/4}} \rightarrow$  turbulent

Chezy's

equation:-

$$\rightarrow V = C \sqrt{mi} = C \sqrt{R \cdot S}$$

$\rightarrow$  where  $v =$  velocity [ $V_{mean}$ ]

$\rightarrow C =$  Chezy's constant  $= \sqrt{\frac{8g}{f}}$

$\rightarrow i(s/s) =$  Hydraulic slope / gradient  $= \left( \frac{h_f}{L} \right)$

$M(O) R =$  Hydraulic mean depth / radius

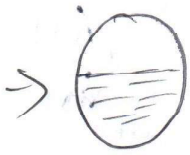
Area of flow / wetted perimeter  $= A/P$

Ex:



Circular pipe running full

$$= A/P = \frac{\pi d^2/4}{\pi d} = \frac{d}{4}$$



Circular pipe - Running half full

$$m = A/P = \frac{(\pi d^2/4)/2}{(\pi d/2)} = \underline{\underline{d/4}}$$

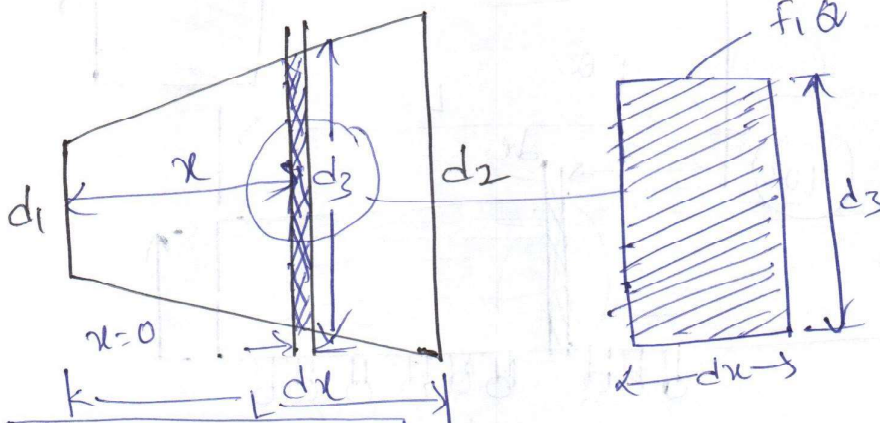
$$\rightarrow V = C \sqrt{m i}$$

$$V^2 = C^2 m i$$

$$V^2 = C^2 \cdot \frac{d}{4} \times \frac{h_f}{L} \Rightarrow V^2 = C^2 \cdot \frac{d}{4} \times \frac{F K V^2}{2g \cdot d} \cdot \frac{1}{K}$$

$$\Rightarrow 1 = \frac{C^2 F}{8g} \Rightarrow C = \sqrt{8g/F}$$

Sp. case 1



$$\rightarrow h_f = \frac{F L a^2}{12 \cdot I d^5}$$

$$d_3 = d_1 + \left(\frac{d_2 - d_1}{L}\right) x$$

$$h_f = \int d h_f = \int \frac{F (dx) a^2}{12 \cdot I (d_3)^5}$$

$$= \frac{F a^2}{12 \cdot I} \int_{x=0}^L \frac{dx}{\left(d_1 + \left(\frac{d_2 - d_1}{L}\right) x\right)^5}$$

$$h_f = \frac{F a^2}{12 \cdot I \left[\left(d_1 + \left(\frac{d_2 - d_1}{L}\right) x\right)^4\right]_0^L} \int_0^L \frac{dx}{x^5}$$

$$h_f = \frac{F Q^2}{12.1 \left[ d_1 + \frac{(d_2 - d_1)}{L} x \right]^5} \int_0^L \frac{dx}{x^5}$$

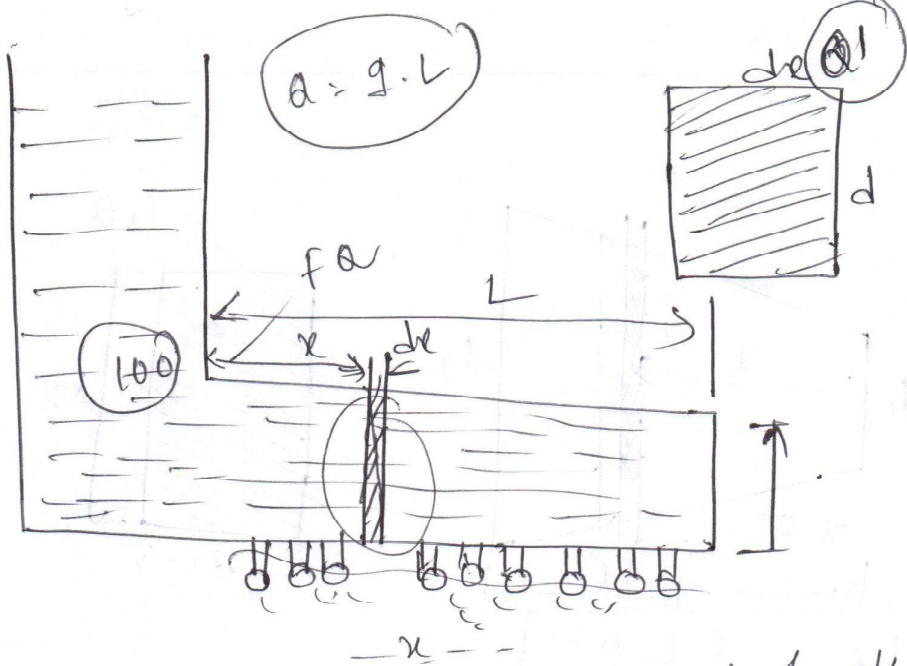
$$\int x^m dx = \frac{x^{m+1}}{m+1}$$

$$\int_0^L x^{-5} dx = \left[ \frac{x^{-5+1}}{-5+1} \right]_0^L = \left[ \frac{x^{-4}}{-4} \right]_0^L = \left[ -\frac{x^{-4}}{4} \right]_0^L$$

$$h_f = \frac{F Q^2}{12.1 \left[ d_1 + \frac{(d_2 - d_1)}{L} x \right]^5} \left[ -\frac{x^{-4}}{4} \right]$$

$$\frac{x^{-5+1}}{-5+1} = \frac{x^{-4}}{-4}$$

Spt. case 2



$\rightarrow q = \text{Lateral discharge per unit length}$

$$h_f = \frac{1}{3} \left[ \frac{F L Q^2}{12.1 d^5} \right]$$

$$a' = a - q \cdot x$$

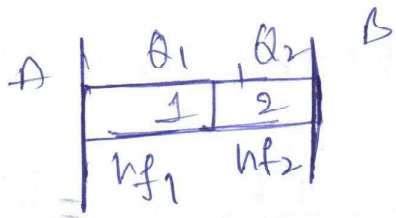
$$a' = q[L - x]$$

$$h_f = \int dh_f = \int \frac{F \cdot (dx) (a')^2}{12.1 d}$$

$$= \frac{F}{12.1 d} \int_0^L [a - qx]^2 dx$$



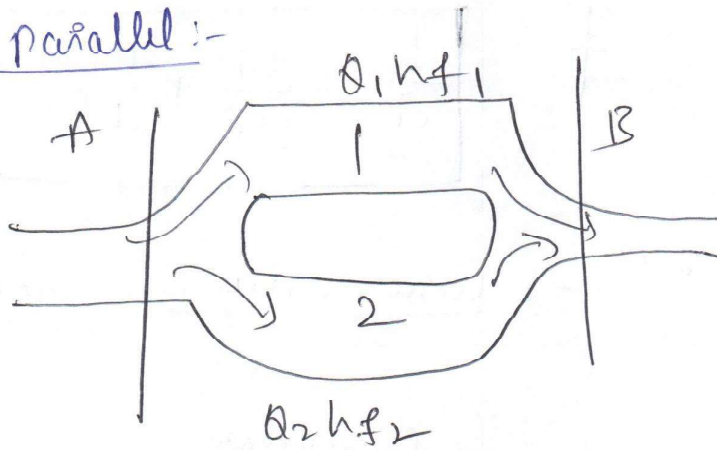
# Pipe Connection Series :-



$$Q_{AB} = Q_1 = Q_2$$

$$h_{fAB} = h_{f1} + h_{f2}$$

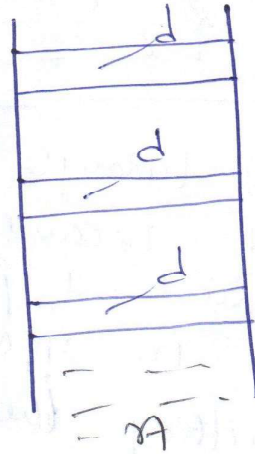
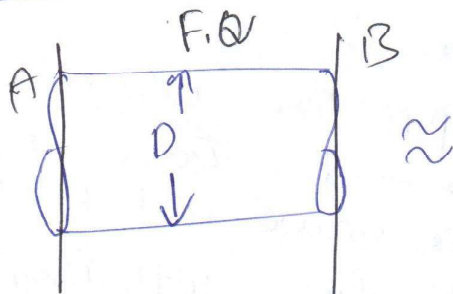
→ parallel :-



$$Q_{AB} = Q_1 + Q_2$$

$$h_{fAB} = h_{f1} = h_{f2}$$

pipes are in Reservoir :- Pipes are in Reservoir



$$d = \frac{D}{n^{2/5}}$$

→ Given head difference

$$h_{fAB \text{ (1)}} = h_{fAB \text{ (n)}}$$

$$\rightarrow h_{fAB \text{ (1)}} = h_{fAB \text{ (n)}}$$

$$\frac{f \cdot L \cdot Q^2}{12 \cdot 10^5} = \frac{f \cdot L \cdot (Q/n)^2}{12 \cdot 1 \cdot d^5}$$

$$\rightarrow \frac{1}{10^5} = \frac{1}{n^2 d^5} \Rightarrow \boxed{d = \frac{D}{n^{2/5}}}$$