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06CV82

Eighth Semester B.E. Degree Examination, May/June 2010
Design and Drawing of Steel Structures

Time: 4 hrs.

Max. Marks:100

- Note:** 1. Answer One question from Part A and One question from Part B.
2. Use of IS:800 – 2007 and steel table are permitted.
3. Assume missing data suitably.

PART – A

- 1 a. A secondary beam ISMB 300 @ 461 N/m is to be joined to the main beam ISMB 400 @ 616 N/m. Two angles ISA 90 x 90 x 6 are used for connection. Three bolts of diameter 20 mm are used to connect angles to the web of ISMB 300. 6 bolts of 22mm diameter are used to connect the angles to the web of ISMB 400. The flanges of both beams are at the same level. Draw to a suitable scale.
- i) Sectional elevation. (08 Marks)
ii) Side view showing all details. (07 Marks)
- b. Two channels ISMC 400 @ 494 N/m placed back to back with a spacing 250 mm. The channels are supported on a slab base having 700 x 700 x 80 mm size. The side angles are 100 x 100 x 10 and are connected by suitable bolts. Base plate is connected to concrete pedestal 2m x 2m x 0.8 m size using 4 anchor bolts of diameter 20 mm having 250 mm length. Draw to a suitable scale the following :
- i) Sectional elevation. (08 Marks)
ii) Plan of slab base giving all details. (07 Marks)
- 2 a. An ISLB 400 @ 569 N/m beam connects ISHB 300 @ 588 N/m column by means of stiffened connection. The stiffener angles are ISA 90 x 90 x 10 mm with two rows of 5 bolts of diameter 20 mm. Seat angle is ISA 130 x 130 x 10 mm and cleat angle ISA 90 x 90 x 10 with nominal bolting. Draw :
- i) Front elevation. (10 Marks)
ii) Side view with details. (10 Marks)
- b. An ISHB 300 @ 630 N/m in the lower storey of a building is connected to ISHB 200 @ 400 N/m of the upper storey. The splice for the columns consists of 250 x 300 x 8 mm plates with all round welding. The base plate is 300 x 250 x 50 mm. Use suitable web angles and welding for connection. Draw:
- i) Sectional Elevation.
ii) Side view with details. (10 Marks)

PART - B

- 3 The fink roof truss for an industrial building has the following details.
Span = 10 m, Rise 2.5 m, bearing = 300 mm, slope $\theta = 26.6^\circ$. Panel width AD = DE
EF = FC = 1.4 m.

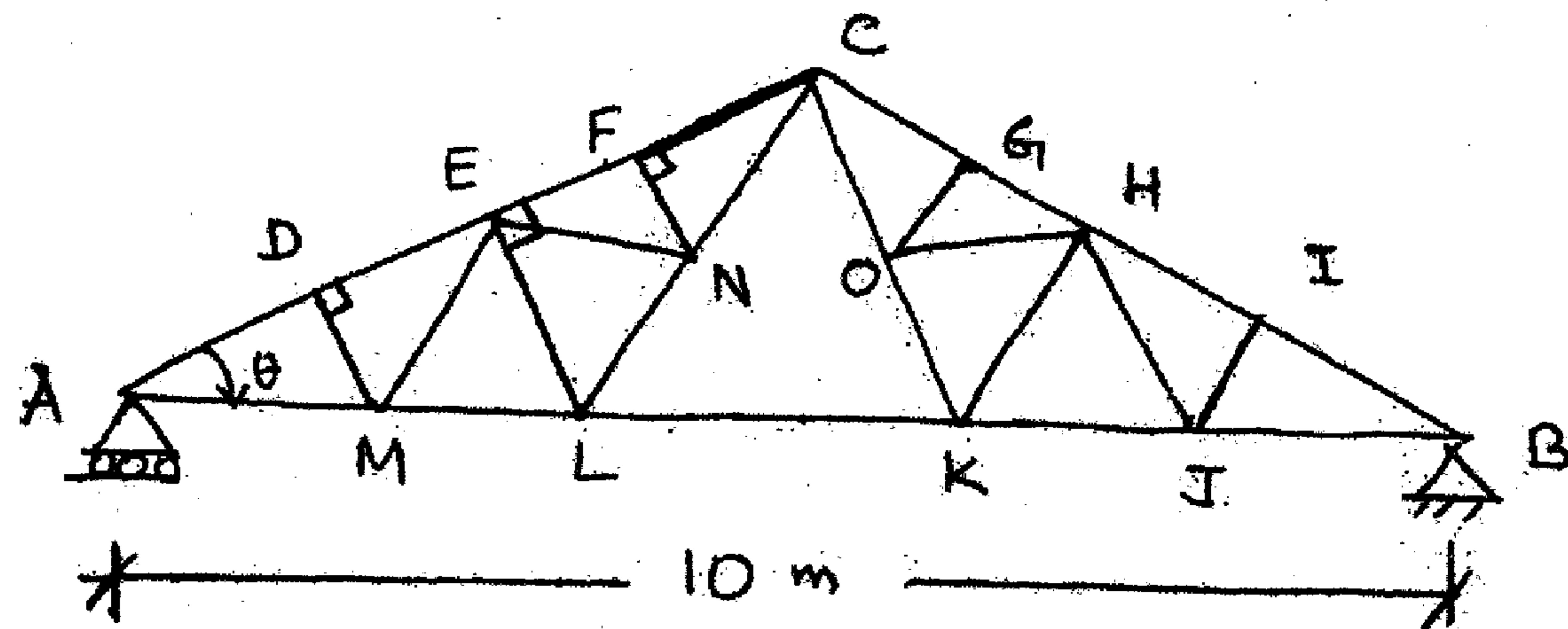


Fig.3

Design the members of the joint C and A completely. Also design the sliding base at A consisting of shoe angles, base plate and bearing plate. The forces under service condition are :

Reaction at A = 20 kN

Force in CF = CG = 36 kN compression and 22 kN tension

Force in CN = CO = 20 kN compression 15 kN tension

Force in AD = 67 kN (compression)

Force in AM = 60 kN (tension).

Use suitable diameter bolts for all connections. Also design the anchor bolts for a pull of 20 kN. Draw to a suitable scale :

- The elevation of roof truss greater than half span.
- Elevation of joint C to a larger scale.
- Elevation and plan details of sliding joint at A.

(70 Marks)

- 4 Design a simply supported crane gantry girder for the following data. The girder is electrically operated. Yield stress of steel is 250 MPa. Use 16 mm diameter bolt of grade 4.6.

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| i) | Span of crane girder | = 20 m |
| ii) | Span of gantry girder | = 7 m |
| iii) | Capacity of the crane | = 220 kN |
| iv) | Self weight of crane excluding crab | = 200 kN |
| v) | Weight of crab | = 60 kN |
| vi) | Wheel base distance | = 3.4 m |
| vii) | Minimum hook approach | = 1.1 m |
| viii) | Self weight of rail | = 0.3 kN/m |
| ix) | Height of rail | = 75 mm |

Draw to a suitable scale :

- The cross section of gantry girder and its attachments to supporting column and the bracket.
- Plan details.
- Side elevation.

(70 Marks)

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